

# Cold In-place Recycling

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2015 Nevada Transportation Conference

April 15, 2015

# Cold In-place Recycling (CIR)

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Distressed Pavement = New Pavement  
Using A Train of Equipment that:

- Mills deteriorated pavement (Typically 2.5 to 4 inches)
  - Reclaimed asphalt pavement (RAP)
- Crushes RAP to gradation
- Mixes with recycling agent
- Re-Paves recycled mix
- Compacts to specified density
- Readies for surface treatment



# Typical CIR Crushing, Sizing and Mixing Train

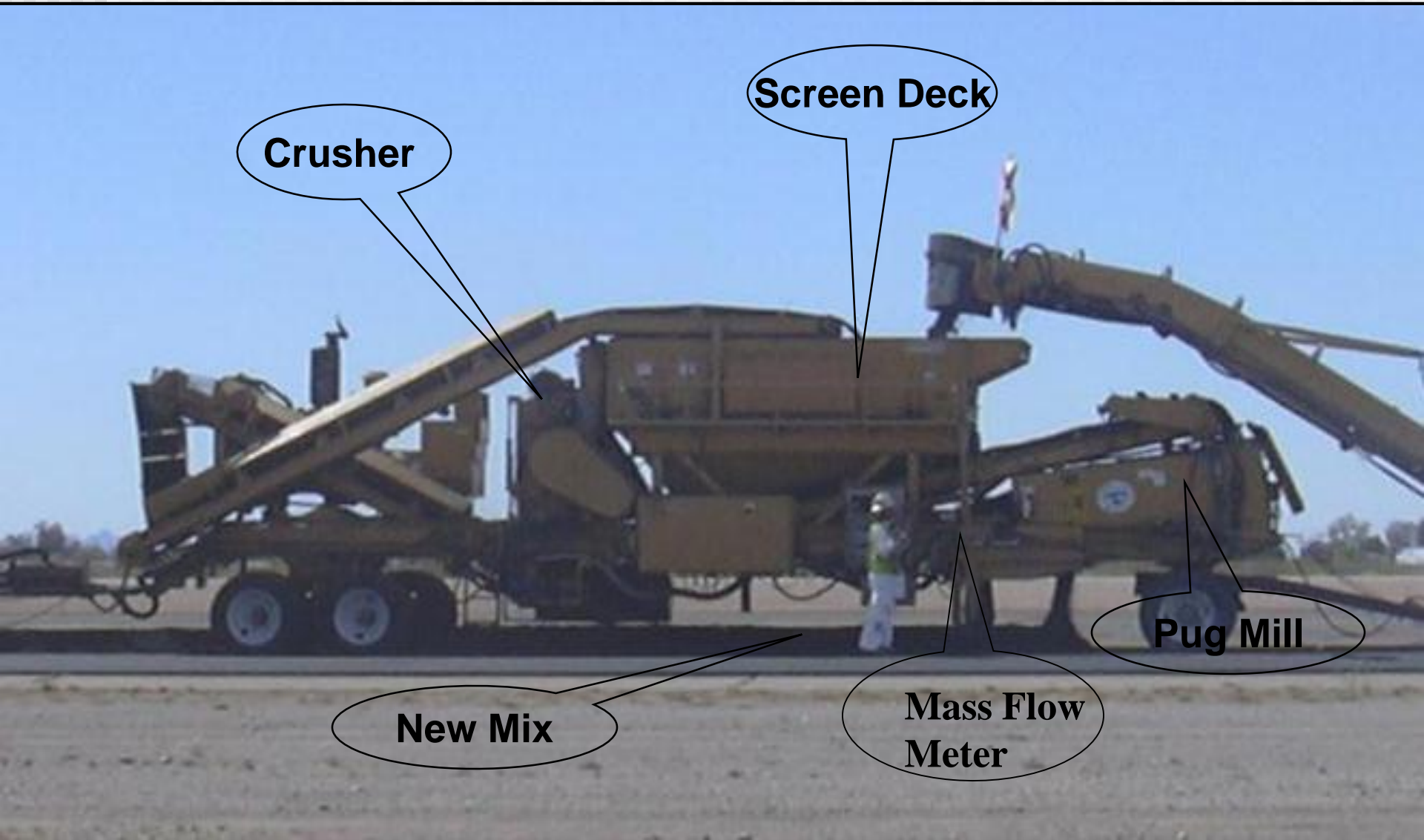
Recycling Unit

Full Lane Mill

Recycling Agent



# Recycling Plant



**Crusher**

**Screen Deck**

**New Mix**

**Mass Flow  
Meter**

**Pug Mill**



# New Recycled Mix Windrowed

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# Pick Up and Paved



# Compacted



## Compacting Equipment

Minimum 1 pneumatic-tired roller at least 25 tons

At least 2 double drum vibratory steel-wheeled rollers at least 10 tons

Minimum width at least 66-inches.

All rollers must have working water spray systems.



# Fog Sealed and Sand Blotted



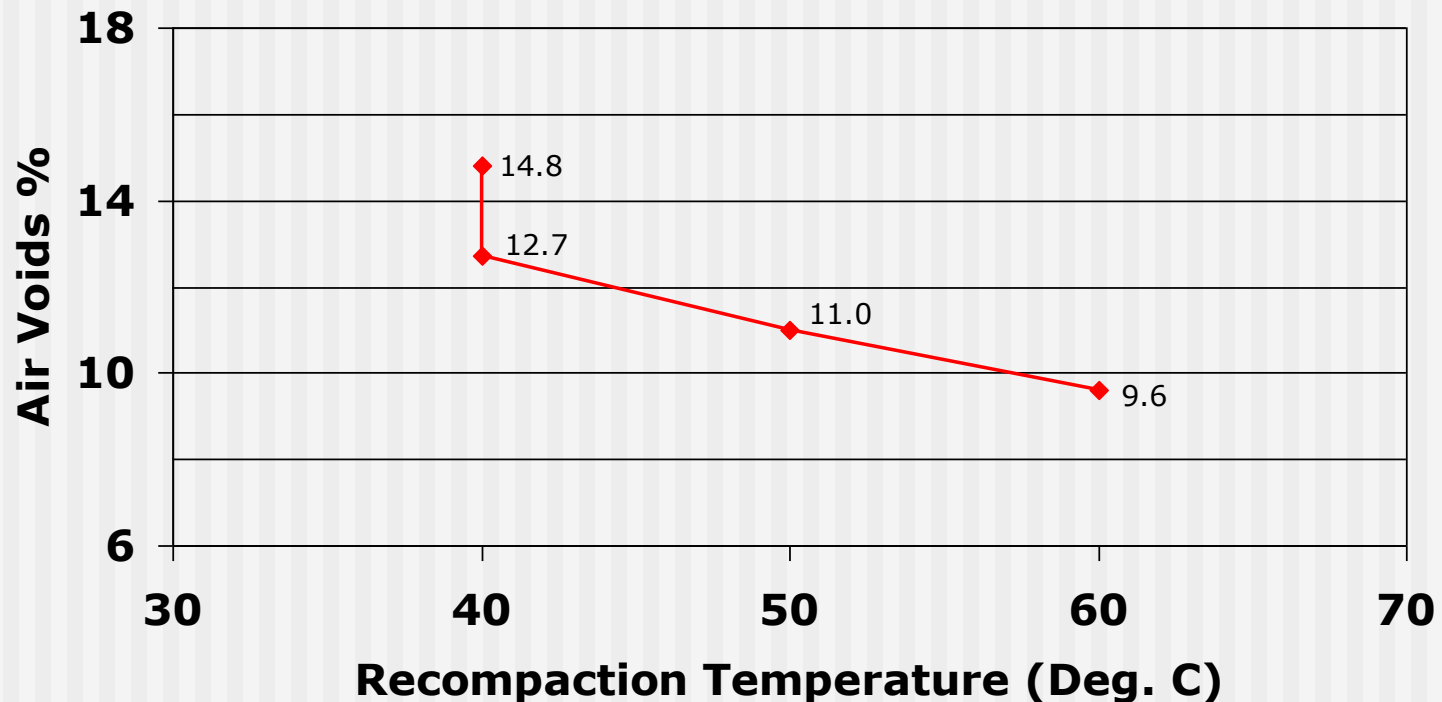


# Open to Traffic



# Supplemental Compaction (Reroll after a few days cure)

Best opportunity to reduce air voids if done properly





A high-angle, close-up photograph of a weathered asphalt road. The surface is covered in a dense network of dark, irregular cracks, creating a mosaic-like pattern. Two parallel yellow lines, likely lane markings, run diagonally from the bottom left towards the top left of the frame. The text "QUALITY PROPER SITE SELECTION" is overlaid in the center in a white, sans-serif font.

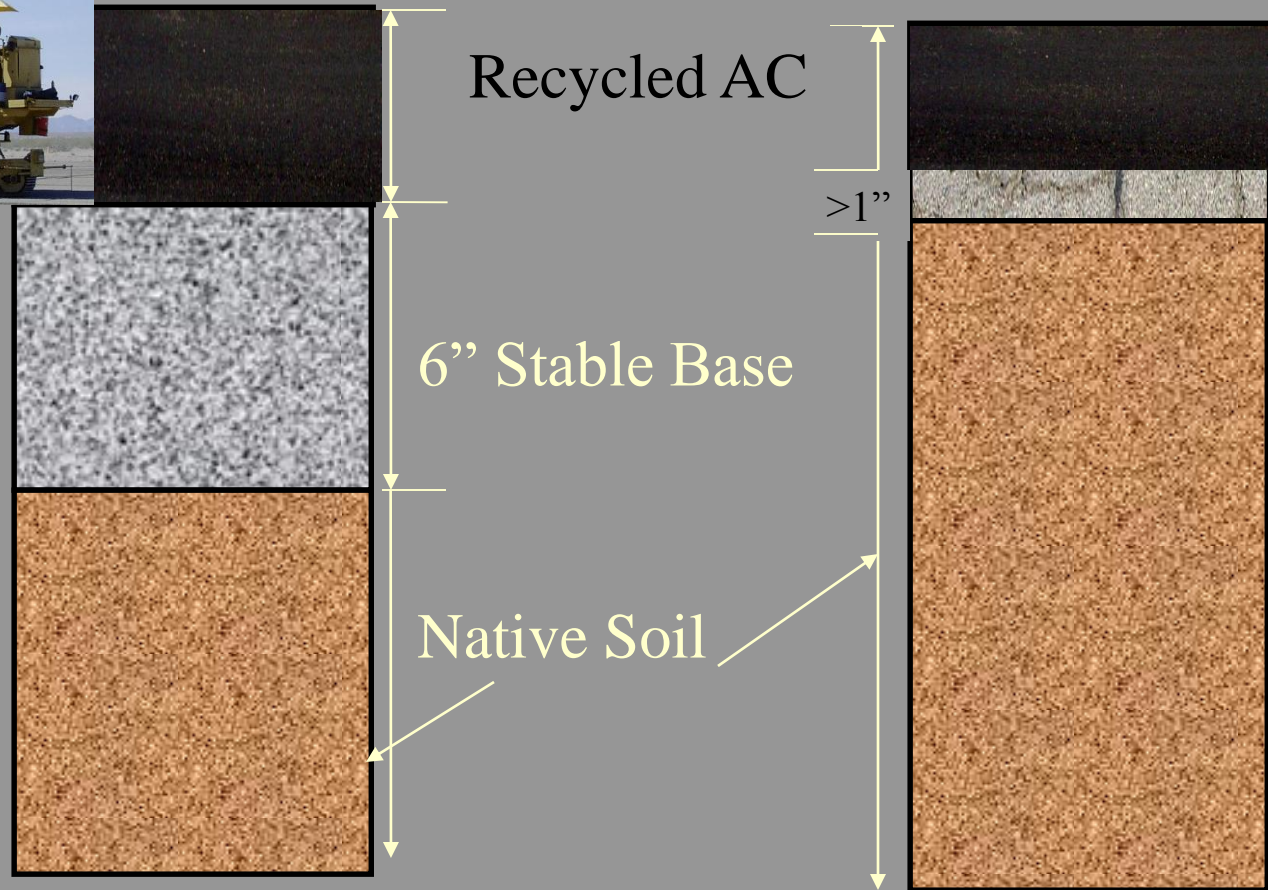
QUALITY  
PROPER SITE SELECTION

# Cold In-place Recycling (CIR) – Preservation/Minor Rehab.



Recycle AC to:

- Stable Base
- Within 1" of less Supportive Material





# Poor Subgrade Will Not be Fixed by CIR

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# Cracking Pattern Disrupted Does Not Need to Go Full Depth

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**70% Rule for Mitigation of Reflective Cracking**



# Pavements with Difficulty to CIR

**Poor Drainage**



***Paving fabric makes it messy!***

**Poor Base**



**Fabric**

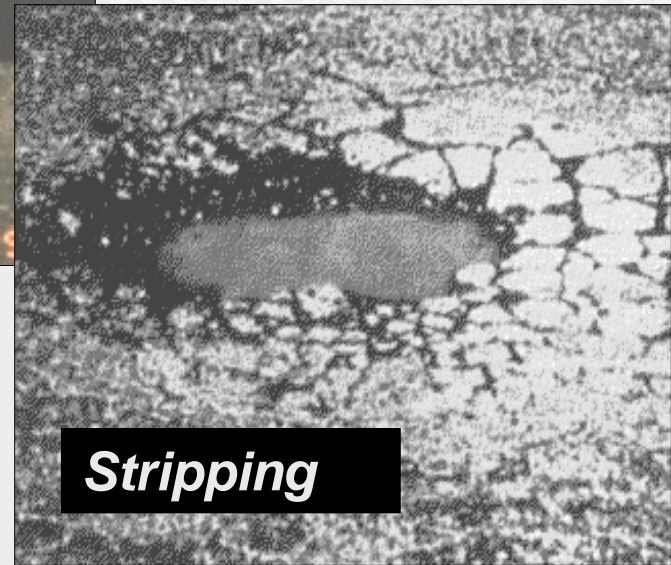


***Asphalt  
Rubber***

***Thin AC and getting into  
the subgrade.***

***Avoid base problems!***

**Stripping**



# Proper Site Selection Roadway Geometry

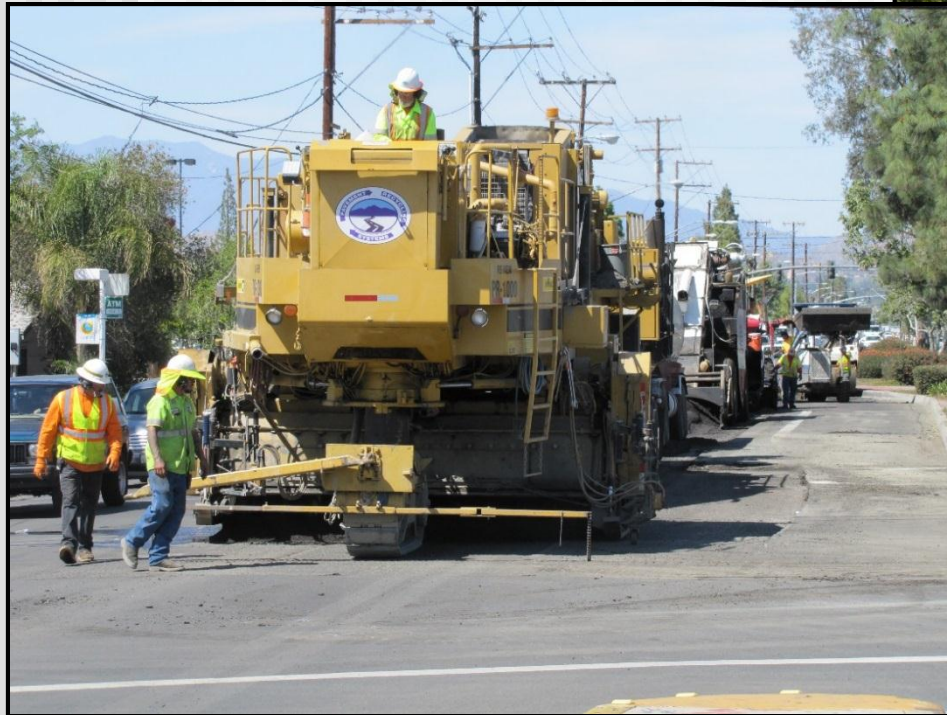
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Main mill 12.5-foot wide cutter. Allows for full lane width and overlap. Close to 14' wide.

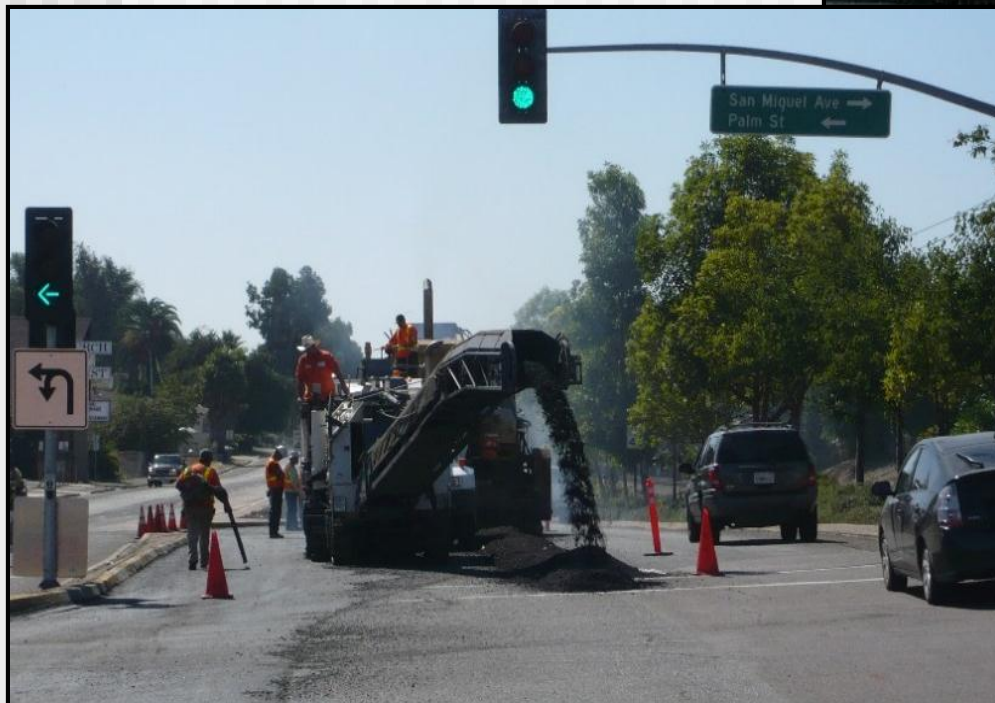


# Does Not Mean Limited to Big Open Highways



# Supplemental Mill

Highway Shoulders to 5' Wide



Urban Areas  
Turning Lanes and Tight Areas



# Proper Site Selection

## Material Balance

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Will swell. Finished voids about 9 to 15%.  
Consider shoulders that are not recycled and changes in grade







QUALITY  
DURING DESIGN



# Structural Design Considerations

## Structural Number

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ARRA BARM II	0.30 – 0.35
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Virginia Center for Transportation Innovation and Research/VDOT Interstate 81 Project	0.35 – 0.39
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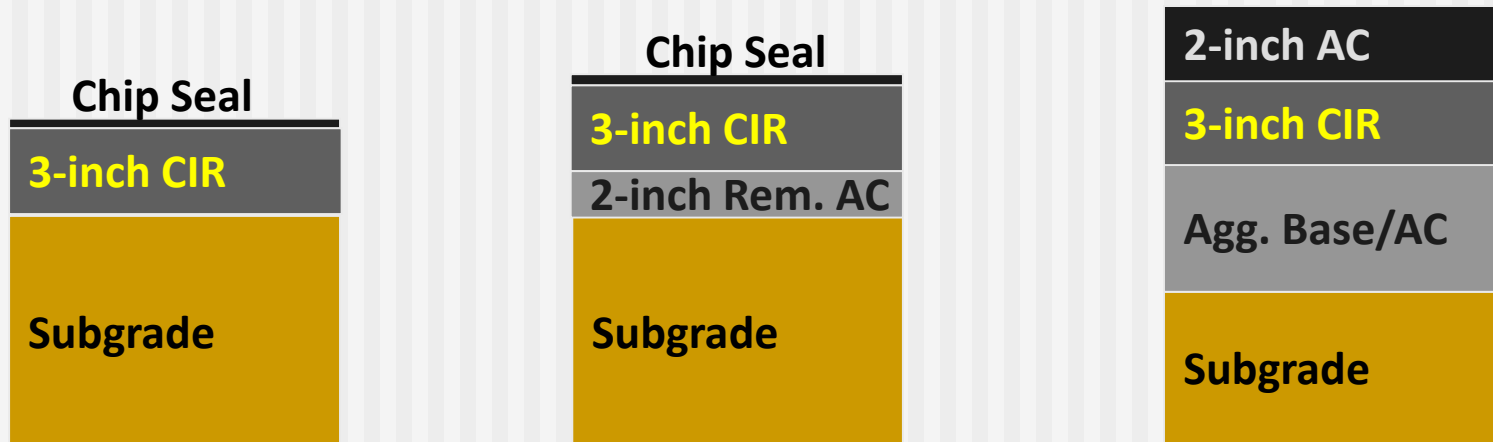
Adaption and Verification of AASHTO Pavement Design Guide - Ontario Department of Ministry	0.28 – 0.38
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NCAT Test Track Testing Cold Central Plant Recycling Based on 10 Million ESALs	0.36 – 0.39
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NCHRP 9-51 - Material Properties for CIR and FDR for Pavement Design	
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# Not All 3" CIR Is The Same

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It's About the Mix and  
How it is Used in the Structural Design



# Structural Design Considerations

## Strength Development

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**Initial Curing** – A short time period that permits the recycled mixture to gain sufficient cohesion to be less susceptible to surface disturbance. Opened to traffic. A few hours.

**Intermediate Curing** – Time required for recycled mixture to build sufficient strength prior to placement of the surface course. Depends on recycling agent and environmental conditions. A few days.

**Final Curing** - Time required for the recycled mixture to reach its ultimate strength. Typically for CIR 6 years.

# Intermediate Curing and Coring

Cures From Top Down



2 Day No Retrieval

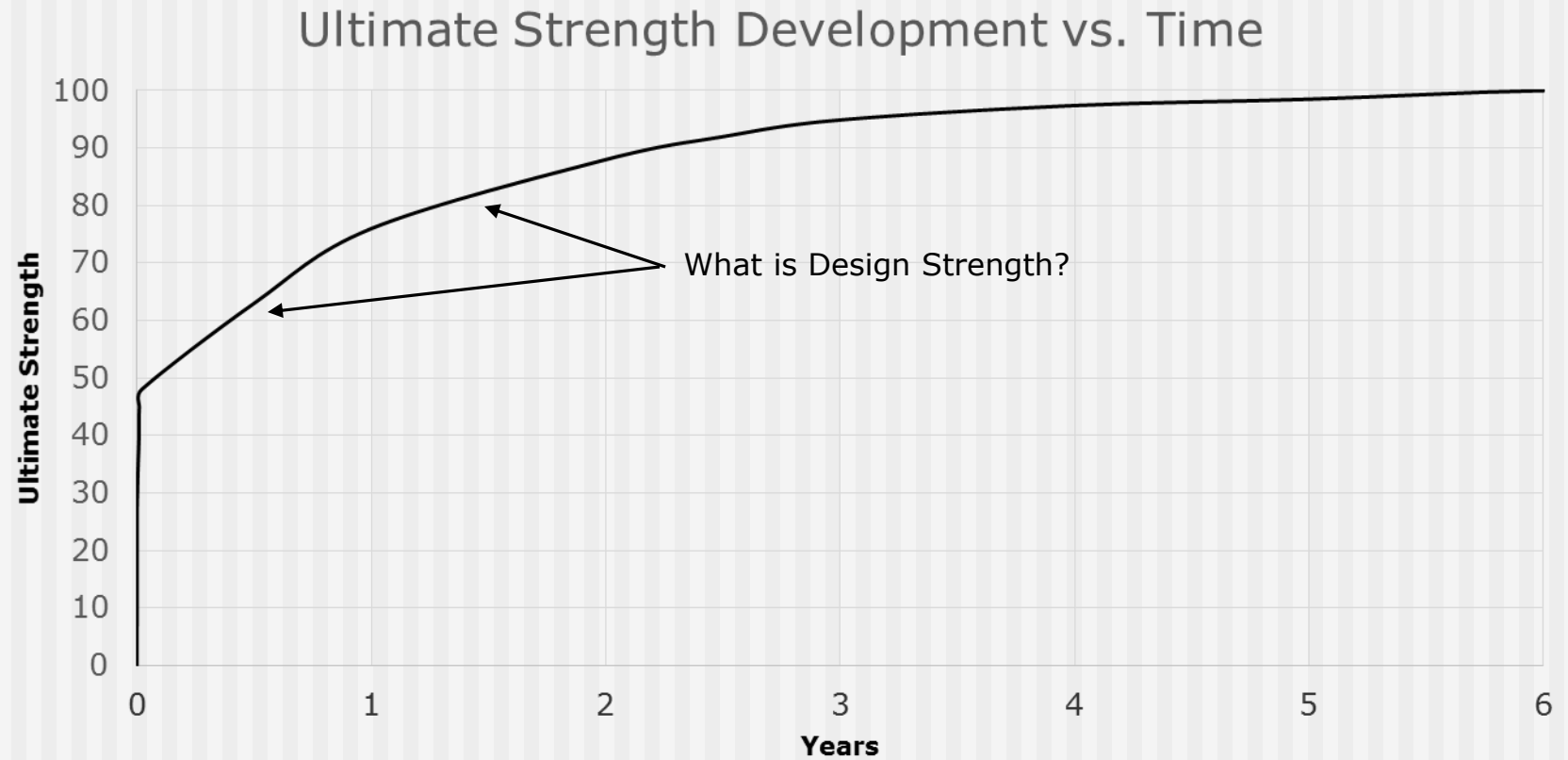
1 Week

5 Weeks

10 Weeks



# Final Curing



# Emery, 2007

LAS VEGAS

ARRA 2007 SEMI-ANNUAL MEETING

OCTOBER 2007

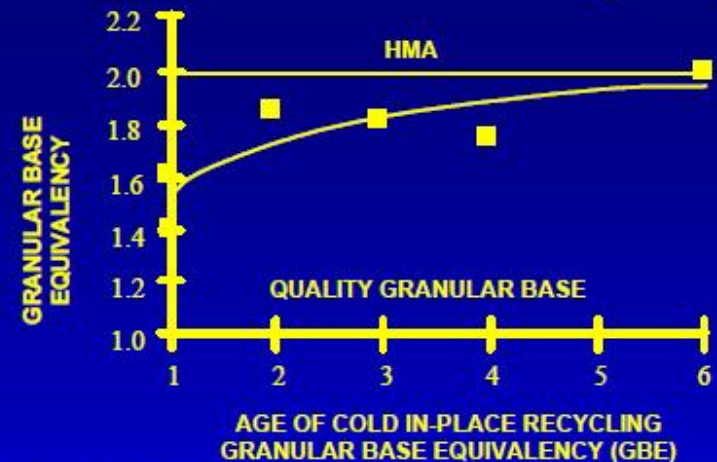
## CIR WITH EMULSION GRANULAR BASE EQUIVALENCY FACTORS (GBE)



TYPICAL CIR STABILIZED BASE CORES



JEGEL NAT



## GBE BASED ON ONTARIO EXPERIENCE

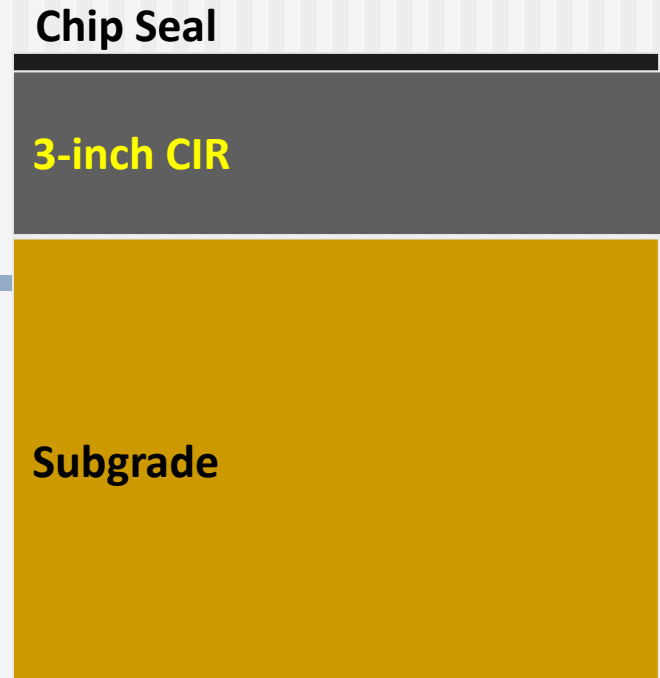
- **NEW PROJECTS**
  - HOT-MIX ASPHALT INCLUDING HIR 2.0
  - GRANULAR BASE (CRUSHED, CBR  $\geq$  60) 1.0
  - GRANULAR SUBBASE (CBR  $<$  60) 0.67
- **RESURFACING PROJECTS**
  - OLD HOT-MIX ASPHALT 1.25
  - OLD GRANULAR BASE 0.75
  - OLD GRANULAR SUBBASE 0.50
  - CIR AND FDR 1.80
- **RECONSTRUCTION PROJECTS**
  - OLD HOT-MIX ASPHALT 1.0
  - OLD GRANULAR BASE 0.6
  - OLD GRANULAR SUBBASE 0.4

CTAA 1996



# Life Just a Few Years?

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## **Yielding Support**

Can Lead to Curing Fatigue

Contamination in Mix – Reduction in Performance

## **Thinner Surface Course**

Less Thermal Insulation – Higher Temperature Impacts

More Environmental Harding

Less Resistance to Traffic Shear Forces

# Underlying Support Critical For Performance

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# Curing Fatigue Cracking

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# Longer Life 6 to 10 Years

Chip Seal

3-inch CIR

2-inch Remaining AC

Subgrade

## Solid Support

Full Strength Development  
No Contamination

## Thinner Surface Course

Less Thermal Insulation – Higher Temperature Impacts  
More Environmental Harding – Less Resistance to Reflective  
Cracking  
Less Resistance to Traffic Shear Forces

## Life Based on Traffic Forces



# Long Life 7-20 Years (Perpetual?)

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Full Strength Development  
No Contamination of Mix

## Surface Course Provides

Thermal Insulation – Less Temperature Impacts  
No Environmental Harding – Resistance to Reflective  
Cracking  
Resistance to Traffic Shear Forces

**Life Based on Proper Structural Design**

2-inch AC

3-inch CIR

Agg. Base/Remaining AC

Subgrade

# NCAT Test Track, 2012

10 million ESALs  
Applied in 2 years  
First cycle completed  
2014  
Anticipate continuing as  
part of 2015 track cycle



## N3

6-inch AC

5-inch CCPR

6-inch Agg Base

Subgrade

## N4

4-inch AC

5-inch CCPR

6-inch Agg Base

Subgrade

## S12

4-inch AC

5-inch CCPR

8-inch FDR

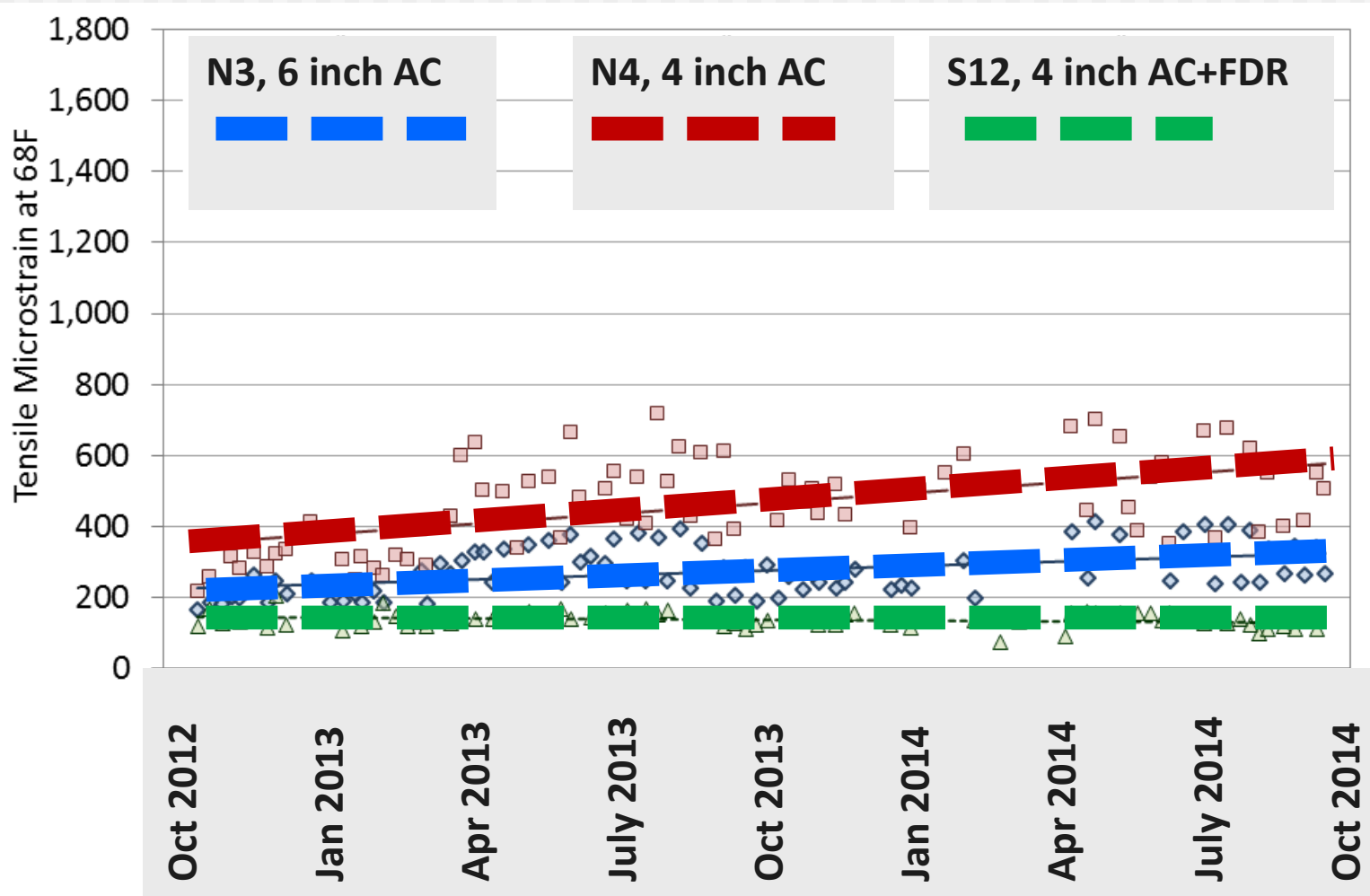
Subgrade

Slide Courtesy of  
Brian Diefenderfer, Ph.D., P.E.  
Virginia Center for Transportation Innovation and Research / VDOT



# NCAT, Lessons Learned

## Tensile Strain Beneath CCPR Layer at 68F



Rutting < 0.25 inches

Will the presence of the FDR section make the CCPR act "perpetually"?

Slide Courtesy of  
Brian Diefenderfer, Ph.D., P.E.  
Virginia Center for Transportation Innovation and Research / VDOT

# Quality Sampling and Mix Design

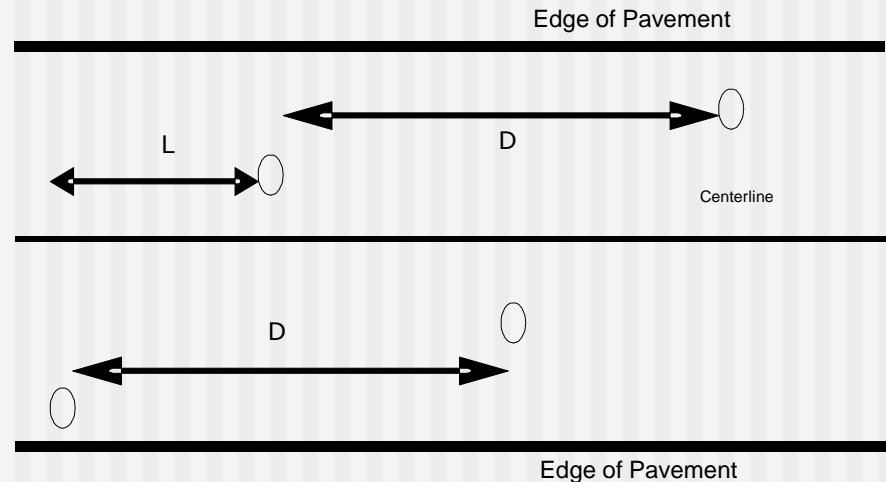




# Coring – For Depth and Use In Mix Design

Cores cut in lab to planned recycling depth and only that portion to be recycled used for mix design

Cores measured to the nearest 1/8-inch (3-mm) and placed in separate containers and labeled





# Dynamic Cone Penetrometer (DCP)



## Addresses Subgrade:

To Support Train

To Support Temporary Traffic

DCP	Acceptable	Marginal	Poor
Each Set of 10 Blows	< 6 Inches <150 mm	6 to 10 inches 150 mm to 250 mm	> 10 Inches > 250 mm
Inches per Blow mm per Blow	0.6 15	0.6 to 1.0 15 to 25	> 1.0 > 25



# Mix Design Current Procedures

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## Performance and Volumetric Testing to Address:

- Gradation and Quality of RAP
- Density and Compaction
- Coating
- Stability and Strength
- Moisture Sensitivity
- Air Voids
- Raveling



# Lab RAP Analysis

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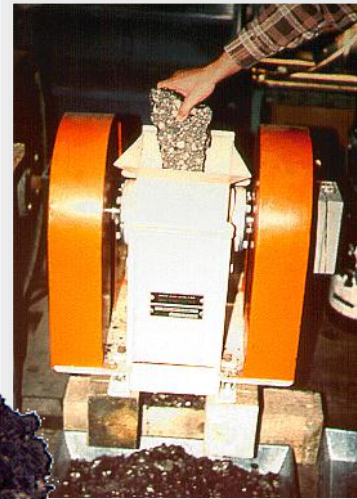
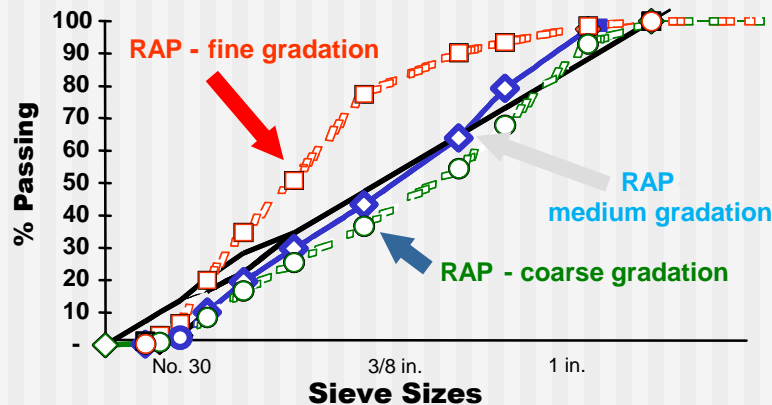
- Extract Binder and Conduct Aggregate Gradation
- Does Parent Gradation Have Sufficient Rock on Rock Stability If Binder Softens Post Construction? What is the Controlling Gradation the RAP or the Parent Gradation?
- Recover Binder
  - Viscosity, Penetration or PG Grading



# RAP Gradation Analysis

## Lab -

Field cores crushed using a lab mill or lab crusher and recombined to specific gradation bands



## Field -

Field gradation depends upon multitude of factors: milling, weather, etc.

Gradation compared to lab tested band

Recycling agent percentage based on applicable gradation

# Additive - Corrective Aggregate

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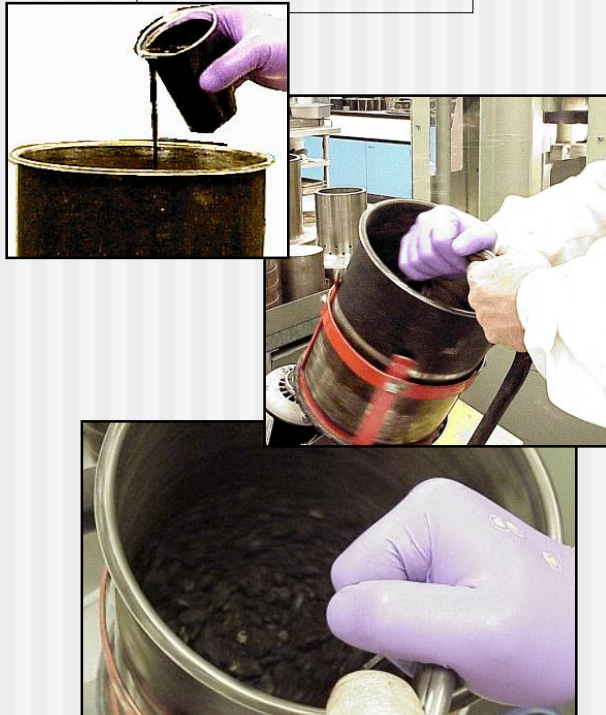
RAP – Better Coating

New Aggregate – Better  
Structure Control

# Mixing With Recycling Agent and Additives

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*Lab*



*Field*



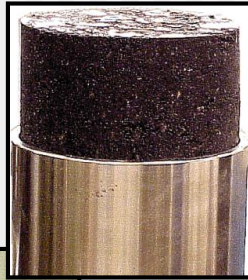
*Pugmill*



# Density Compaction Effort

Superpave Gyrotory Compactor

Lab



Field



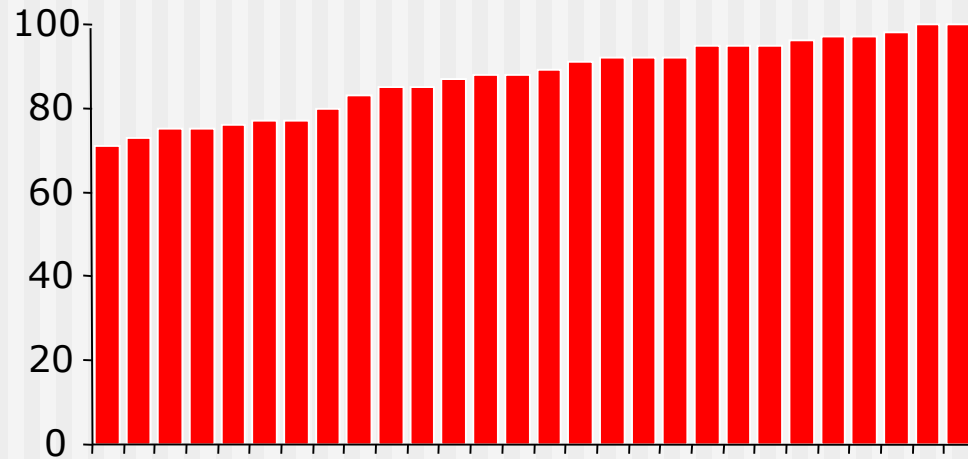
# Coating

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# Test for Stability and Moisture Sensitivity at Appropriate Temperatures by Marshal Stability and/or ITS.

**Retained  
Strength  
%**



■ **Typical CIR Projects**

Chemical Additive to Improve?

Freeze Thaw Test May be Appropriate





# Lime Slurry or Hydrated Powder



Typically 1 to 1.5%

Commonly with a  
solvent based emulsion  
such as CMS2s

Has been used with  
Engineered Emulsions



# Dry Cement

Typically 0.3 to 1.0%



# Chemical Additives

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Can Provide Early Strength and Hardening

Avoid Changing From Ductile to Brittle Behavior

Consider Depth of Recycling and Underlying Support

- Thin/Thick CIR

- Ductile/Rigid CIR

- Flexible Support vs Rigid Support

Maintain Residual Asphalt to Additive Ratio

- Cement 3:1

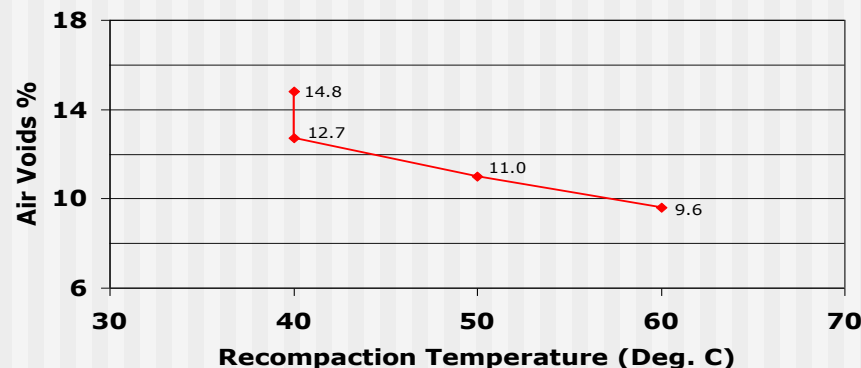


# Air Voids

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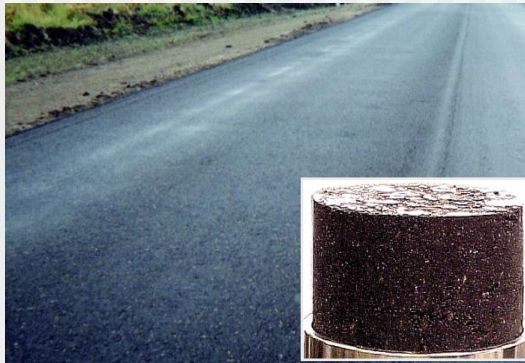
Potential to Reduce Post Construction  
Future Temperatures  
Softening of RAP

How to Decrease  
Mineral or Chemical Fillers  
Higher Binder Contents  
Good Supplemental Compaction Procedures



# Raveling Test

**Proper CIR**



**Inadequate CIR**



Compacted at 77°F Tested at 50°F and 50% Humidity

Does That Represent Construction Conditions/Long Term Performance?

# In Summary Let's Get It Right!

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Due Your Homework Before Construction to Avoid Surprises

Good Recycling Contractors Want Stringent Quality Standards and Specifications

More Agency Successes Lead to More Successful Contractors

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