

University of Nevada, Reno Western Regional Superpave Center **Program and Research Updates** 

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### 2014 Nevada Transportation Conference

April 8-9, 2014 – Texas Station Hotel – Las Vegas, Nevada







Pavements/Materials Engineering Program Western Region Superpave Center

#### Personnel

- Peter E. Sebaaly, Ph.D., P.E. Professor, Director of WRSC
- Elie Y. Hajj, Ph.D.

Assistant Professor

- Mena Souliman, Ph.D.

Post-doctoral Scholar

 Murugaiyah Piratheeban, MSCE Research Scientist, Lab Manager

- Students

15 Research Graduates: 5 Undergraduates

### Facility

- WRSC occupies parts of the 1<sup>st</sup> & 2<sup>nd</sup> floors of HREL building.
- Fully accredited & certified Lab by AMRL.
- Current capabilities:
  - aggregates
  - asphalt binders
  - asphalt mixtures
  - Portland cement concrete
  - Large-scale pavement testing (flexible and rigid pavements)







## Main Areas of Research Topics







# **Selected Research Studies**

- 1. Implementation of AASHTO MEPDG for Flexible Pavements in Nevada (NDOT).
- 2. Long-Term Performance of CIR in Nevada (NDOT).
- 3. Development of Percent within Limit Specifications (PWL) for Nevada (NDOT).
- 4. Cost-Effectiveness and Optimum Application of Slurry Seal (Washoe RTC).
- 5. Quantifying the Influence of Geosynthetics on Pavement Performance (NCHRP).







## Updates on the Implementation of AASHTO MEPDG for Flexible Pavements in Nevada (Sponsor: Nevada DOT)





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# **Motivation**

## -Why M-E Design?

- Greater emphasis on performance
- Increased emphasis on rehabilitation strategies
- Variations in Climate
- Increased Traffic
- Budgetary constraints
- Nonstandard Materials
- Darwin (AASHTO 93) no longer supported





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# Pavement ME Design for Flexible Pavements

- Prediction of following distresses with time:
  - Rutting
  - Fatigue cracking
  - Transverse cracking
  - Roughness (IRI)









# Why Local Calibration for Nevada?



- Nevada's use of polymer-modified asphalt binder.
- Nationally calibrated performance models are calibrated based on neat asphalt binder only.
- Using the national models will show an early failure compared to the true performance of PM binders.
- This may result in unnecessarily thick sections.







# **Implementation Plan**

- 1. Sample asphalt mixtures from field projects.
- 2. Develop materials database.
- 3. Identify existing climatic weather data.
- 4. Identify existing traffic data.
- 5. Convert NDOT PMS distress data.
- 6. Calibrate to Nevada's Conditions.
- 7. Validate the calibrated models.
- 8. Conduct trainings





# Map of Sampled Contracts (2005-2010)





## **Candidate Sections for Calibrations**

Fatigue Sections											
	Sections			Sampled	Non Sampled	Age(years)	Sampled	Non	Fatigue (%)	Sampled	Non
	Non Sampled	Sampled		Sampled Non Sampled	Sampled			Sampled			
PG64- 28NV	5	0	Low <1000	0	1	0 to 3	0	1	3 to 10	0	3
			Intermediate 1000-5000	0	1	3 to 6	0	1	10 to 30	0	0
			High >5000	0	3	6 and up	0	3	over 30	0	2
PG76- 22NV	13	2	Low <1000	0	6	0 to 3	0	0	3 to 10	1	8
			Intermediate 1000-5000	2	7	3 to 6	2	3	10 to 30	1	3
			High >5000	0	0	6 and up	0	10	over 30	0	1

Rutting Sections											
	Sections			Sampled	Non Sampled		Sampled	Non	Rutting	Sampled	Non
	Non Sampled	Sampled	Trailic AADTT	Sampleu	Non Sampled	Age(years)	Sampleu	Sampled	Rating(in)	Sampleu	Sampled
DOCA			Low <1000	3	4	0 to 3	6	1	0.5 to 1	9	7
28NV	11	10	Intermediate 1000-5000	7	4	3 to 6	4	5	0.1 to 0.15	1	2
			High >5000	0	3	over 6	0	5	0.15 and up	0	2
PG76- 22NV	15	5	Low <1000	0	6	0 to 3	0	0	0.5 to 1	5	1
			Intermediate 1000-5000	3	7	3 to 6	4	5	0.1 to 0.15	1	14
			High >5000	2	2	6 and up	1	10	0.15 and up	0	0

IRI Sections										
Sections			Sampled	Non Sompled		Sampled	Non	IRI Rating	Sampled	Non
Non Sampled	Sampled		Sampleu	Non Sampled	Aye(years)	Sampleu	Sampled	(in/mile)	Sampleu	Sampled
6	4	Low <1000	1	4	0 to 3	2	0	70 to 90	3	4
		Intermediate 1000-5000	3	2	3 to 6	2	2	90 to 120	1	2
		High >5000	0	0	6 and up	0	4	120 and up	0	0
12	5	Low <1000	0	4	0 to 3	0	0	70 to 90	2	8
		Intermediate 1000-5000	3	6	3 to 6	4	4	90 to 120	3	2
		High >5000	2	2	6 and up	1	8	120 and up	0	2
	Section Non Sampled 6 12	Sections       Non Sampled     Sampled       6     4       12     5	Sector         Traffic AADTT           Non Sampled         Sampled         Traffic AADTT           6         4         Low <1000	IRI       Section     Traffic AADTT     Sampled       Non Sampled     Sampled     1       6     4     Intermediate 1000-5000     3       6     4     Intermediate 1000-5000     3       12     5     Intermediate 1000-5000     3       12     5     Intermediate 1000-5000     3	$ \begin{array}{ c c c } \hline IRI \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$	$ \begin{array}{c c c c c } \hline IRI \ Sections \\ \hline Section \\ \hline Non Sampled \hline Sampled \\ \hline Non Sampled \hline Sampled \\ \hline 1 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \\$	IRI SectionsSectionsNon SampledSampledNon SampledAge(years)SampledNon SampledSampled140 to 3264Low < 1000	IRI SectionsSectionsSectionsSectionsNonSectionsNonSectionsNonNonSectionsSectionsNonSectionsNonSectionsSectionsNonSectionsNonSectionsSectionsNonSectionsNonSectionsSectionsSectionsNonSectionsSectionsNonSectionsSectionsSectionsSectionsSectionsNonSectionsSectionsNonSections <th< td=""><td>IRI SectionsSectionsSectionsNon SampledNon SampledNon SampledNon SampledNon SampledNon SampledNon SampledIRI Rating Cin/mile)Non SampledSampled140 to 32070 to 9064Intermediate 1000-5000323 to 62290 to 12010High &gt;5000006 and up04120 and up125Intermediate 1000-5000363 to 64490 to 120125High &gt;5000226 and up18120 and up</td><td><math display="block"> \begin{array}{c c c c c c c c c c c c c c c c c c c </math></td></th<>	IRI SectionsSectionsSectionsNon SampledNon SampledNon SampledNon SampledNon SampledNon SampledNon SampledIRI Rating Cin/mile)Non SampledSampled140 to 32070 to 9064Intermediate 1000-5000323 to 62290 to 12010High >5000006 and up04120 and up125Intermediate 1000-5000363 to 64490 to 120125High >5000226 and up18120 and up	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $



# Preliminary Calibration for Rutting Model

 Based on 7 pavement sections tested for rutting in the Repeated Load Triaxial.









# Long-Term Performance of CIR Technique in Nevada (Sponsor: Nevada DOT)





 NDOT has used CIR over 1,500 centerline miles of roads over the past two decades (~25% of total system in NV).







- NDOT has long been using CMS-2s for CIR projects and recently started using Reflex and PASS emulsion.
- NDOT has observed some differences in the performance of CIR roads throughout Nevada.
- Need to assess the long-term performance of CIR roads in Nevada.





- Total of 66 CIR projects were evaluated – Construction date 2001-2009.
- Factors Considered:

CIR Rehab Type	<ul><li>With Asphalt Overlay</li><li>With Surface Treatment</li></ul>
Emulsion Type	<ul><li>CMS-2s</li><li>Reflex</li><li>PASS</li></ul>
Geographic Location	<ul><li>District</li><li>County</li></ul>
Traffic Level	- ESALs
Pre-CIR Pavement Condition	- Pavement Distresses





- CIR + HMA Overlay + Surface Treatment
  - CIR with CMS-2S Emulsion
  - CIR with Reflex<sup>TM</sup> Emulsion (2 Projects)
  - CIR with PASS Emulsion
- CIR + Surface Treatment
  - CIR with CMS-2S Emulsion
  - CIR with Reflex<sup>TM</sup> Emulsion
  - CIR with PASS Emulsion

(2 Projects)

(37 Projects)

(15 Projects)

(9 Projects)

(2 Projects)





# Average Effective Life for All Projects Combined





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## Pavement Performance Charts Example: CIR + Surface Treatment



## Pavement Condition Index (PCI) According to ASTM D6433



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# **Findings and Conclusions**

- Transverse cracking & longitudinal cracking were the major type of distresses.
- CIR pavements with HMA overlay significantly reduced rutting and roughness while the other method fairly reduced it.
- Factors found insignificant on the performance of CIR pavements:
  - Variation in environmental conditions
  - CIR layer thickness between 2 and 3 inches
  - Various surface treatments
- Thickness of the overlay was crucial for the performance of CIR on high volume roads.
- CIR + HMA Overlay + Surface Treatment on high volume roads performed better than CIR + Surface Treatment on low volume roads.
- <u>CIR with HMA overlay</u>: Within the first two years, the Reflex and PASS emulsions showed slightly lower performance as compared to the CMS-2S emulsion.
- <u>CIR with surface treatment</u>: all three types of emulsions (CMS-2S, Reflex, and PASS) showed similar performances.





# Pavement Construction: Percent Within Limit Specifications (Sponsor: Nevada DOT)







## Develop PWL System for NDOT (PWL = the percent of a lot falling within set specification limits)

#### Proposed Specs:

PWL process considers both the actual value of the measured property and its associated variability Versus

#### Current Specs:

Pass/No pass specifications







## **Develop PWL System for NDOT** Example



## **Research Phases**

#### • Phase I – Review of Existing PWL Specification Systems

- Literature review
- General framework for the development of the PWL system was recommended for Phase II

#### • Phase II – Develop the PWL Specification System

- PWL system was developed including several materials and mixtures properties to identify the PWLs for all sublots and lots of HMA mixtures
- Weight factors are identified for each of the mixtures properties leading to the development of a single PWL for each lot within a construction project

#### • Phase III – Implement the Specifications

- Implement the developed PWL system on several NDOT projects
- Use the data to fine tune the system as needed





# Weight Factors, Overall PWL, & Pay Factors

- Weight factors determined based on the findings from the NDOT study on "Impact of Construction Variability on Pavement Performance."
  - Gradation:
    Asphalt Binder Content:
    Compaction (i.e., Mat Density):

    25%
    33%
    Herformance-Related Weight Factors

 $PWL_{Overall} = 0.25 PWL_{Gradation} + 0.33 PWL_{AC} + 0.42 PWL_{Compaction}$  $PF = 55 + (0.5 \times PWL_{overall})$ 

A <u>100% pay</u> will be provided to the contractor at a <u>PWL of 90%</u>. Maximum PF will be at <u>105%</u>.







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## Develop PWL System for NDOT Proposed Implementation Plan

Year 2014Apply PWL system on pilot projects.		Pay factors will not be implemented.				
Year 2015	The 100% pay will be provided at an overall PWL of 70.	The maximum pay factor is fixed at 100%.				
Year 2016	The 100% pay will be provided at an overall PWL of 80.	The maximum pay factor is fixed at 100%.				
Year 2017	The 100% pay will be provided at an overall PWL of 90.	The maximum pay factor is fixed at 105%.				







# Cost-Effectiveness and Optimum Application of Slurry Seal (Sponsor: Washoe County RTC)







### **Effectiveness of Slurry Seal** *Problem Statement and Objective*

- Time of application is left to the Project Engineer judgment and practice:
- Identify the optimal timing for the application of slurry seal on asphalt pavements in the Truckee Meadows Region.
  - Phase I: <u>Single application</u> of slurry seal.
  - Phase II: <u>Two sequential application</u> of slurry seal.





## Effectiveness of Slurry Seal Research Plan

- Flexible Pavements: New vs. Overlay Construction.
- Road Class: Arterial, Collector, Residential.
- Performance measured in terms of PCI (0-100).
  - Do-Nothing: SS was not applied to the pavement.
  - Phase I: Single Application
    - SS applied immediately after construction (referred to as 0).
    - SS applied at either: 1, 3, 5, 7, or 9 years after construction.
  - Phase II: Two sequential Application
    - First SS at either 0, 1, 3 or 5 years / Second SS at either 7 or 9 years.





# **Slurry Seal Applications (Example)**





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### Phase I: Single Application of Slurry Seal SS Effectiveness







## **Effectiveness of Slurry Seal** Summary of Findings

- Application of SS <u>immediately or one year after</u> construction is <u>not effective</u>.
- In general, the <u>pavement service life</u> was not extended by <u>application of the single slurry seal</u>.
- Optimum timing for sequential application:



→ Pavement service life extended by 2.0~4.0 years



### **Optimum Timing for Slurry Seal Application** Overall Recommendations

#### **PCI** Rating

Construction	Recommended Threshold PCI Values					
Туре	1 <sup>st</sup> Slurry Seal Application	2 <sup>nd</sup> Slurry Seal Application				
New	87-90	86				
Overlay	85-87	77				









## 01-50: Quantifying the Influence of Geosynthetics on Pavement Performance (Sponsor: National Cooperative Highway Research Program)









# NCHRP 01-50: Research Objective

- To develop a methodology for quantifying the *influence of* geosynthetics on pavement performance for use in pavement design & analysis.
  - Be consistent with the MEPDG framework to facilitate incorporation into the MEPDG
  - Be concerned with using geosynthetics in unbound base/subbase layers for flexible & rigid pavements





### NCHRP 01-50:

#### Large Scale Experiment for Flexible and Rigid Pavements

- 1. Assess vertical pressure distributions above/below geosynthetic layer;
- 2. Assess tensile stress/strain distribution within geosynthetic;
- 3. Assess deformed shape of geosynthetic;
- 4. Assess confinement of materials provided by geosynthetic;
- 5. Assess slippage condition at the geosynthetic/unbound material interface.





## NCHRP 01-50: Subgrade Preparation and Placement



Subgrade Compaction

**Completed Subgrade Placement** 





## NCHRP 01-50: Base Instrumentation



#### Subgrade Pressure Cell



#### Excavated Base Area for Instrumentation





## NCHRP 01-50: Surface Instrumentation



#### Loading Actuator and Plate





# Thank You!







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